VEERMATA JIJABAI TECHNOLOGICAL INSTITUTE

[Central Technological Institute, Maharashtra state] Matunga, Mumbai-400019

SEMESTER EXAMINATION SEMESTER & COURSE TIME ALLOWED SUBJECT(Code):

MAY 2012 IV SYBTECH-(CIVIL) 3HRS DATE OF EXAM: 14/05/12 Time: 1:30 To 4:30

SOIL MECHANICS (0225)

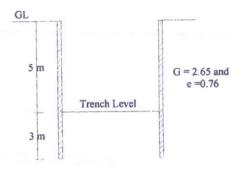
MAX MARKS: 100

Instructions:

- 2. All questions are compulsory.
- Figures to the right indicate full marks.
- Assume suitable data if necessary.
- Illustrate your answers with neat sketches wherever necessary.
- Q.1 Answer the following
 - Derive the following relationship

 $\gamma_{sat} = \gamma_w \left(\frac{G+e}{1+e} \right)$ and $\gamma_d = \frac{G\gamma_w}{1+e}$ with usual notations.

- b. Compare energies given in standard proctor and modified proctor tests. 4
- Calculate time required in years for 90% consolidating a clay layer of 5 m 4 depth surrounded by sandy silt. The Cv value is 0.000256 cm²/s.
- d. In shear test if $\alpha = 50^{\circ}$, $\sigma = 20$ kPa, c = 10 kN/m2. Determine ϕ and shear 4 strength.
- e. If k_1 is the permeability at e_1 void ratio. Determine the k_2 at void ratio e_2 .
- Q.2 a. Discuss field identification methods for fine grained soil.
 - b. An undisturbed specimen of clay has volume of 18.9 cm³ and mass of 30.2 8 g. On oven drying mass reduces to 18 g. The mass mercury displaced by dry soil pat 134.64 g. Determine the shrinkage limit, specific gravity shrinkage ratio and volumetric shrinkage.
 - c. Can clayey soil undergo quick condition? Check the stability against quick 6 condition of side support as shown in Fig.1. If yes what is the remedy.



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- Q.3 Show that average permeability (k_x) parallel to bedding plane is greater than average permeability perpendicular (k_v) bedding plane. Write any one 6 field determination of permeability test.
 - Write applications of flow net.

Draw dry density moisture content curve and find out MDD and OMC tested 10 C. in standard proctor test using small mould (1000 cc capacity) and light hammer. Also find maximum theoretical density at w=15.5%, if G =2.7

| Compacted wt of soil, | 1800 | 1940 | 2000 | 2005 | 2003 | 1980 |
|-----------------------|---------|------|---------|------------|--------|------|
| gm | torhits | 1000 | EU STEW | Control of | E DOIL | |
| Water content, % | 8.5 | 12.2 | 13.75 | 15.5 | 18.2 | 20.2 |

Derive the relationship between major and minor principle stresses. Modify 6 Q.4 a. the same for unconfined compression strength condition.

Evaluate effective shear parameters from the results of CU test b.

| Cell pressure, kPa | 150 | 300 | 450 | 600 |
|--------------------------|-----|-----|-----|-----|
| Deviator stress, kPa | 102 | 200 | 304 | 405 |
| Pore water pressure, kPa | 80 | 164 | 264 | 325 |

Explain types of slope failure in embankment.

Q.5

a.

6

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- Discuss any one graphical method of evaluation of C_v . A clay soil tested in consolidation cell showed void ratio decreases from 1.2 10 b. to 1.10 when the pressure was increased from 0.25 to 0.50 kg/cm². Calculate coefficient of compressibility, coefficient of volume change. If coefficient of consolidation is 10 m²/yr of a 5 m thick sample surrounded by sandy deposits, evaluate the time required for 90% consolidation. Also evaluate the settlement by using $\Delta H/H = H(\Delta e/1+e)$.
- Write symbols with description of fine grained soil. Write steps to classify 6 SP-SM according to I.S. classification.